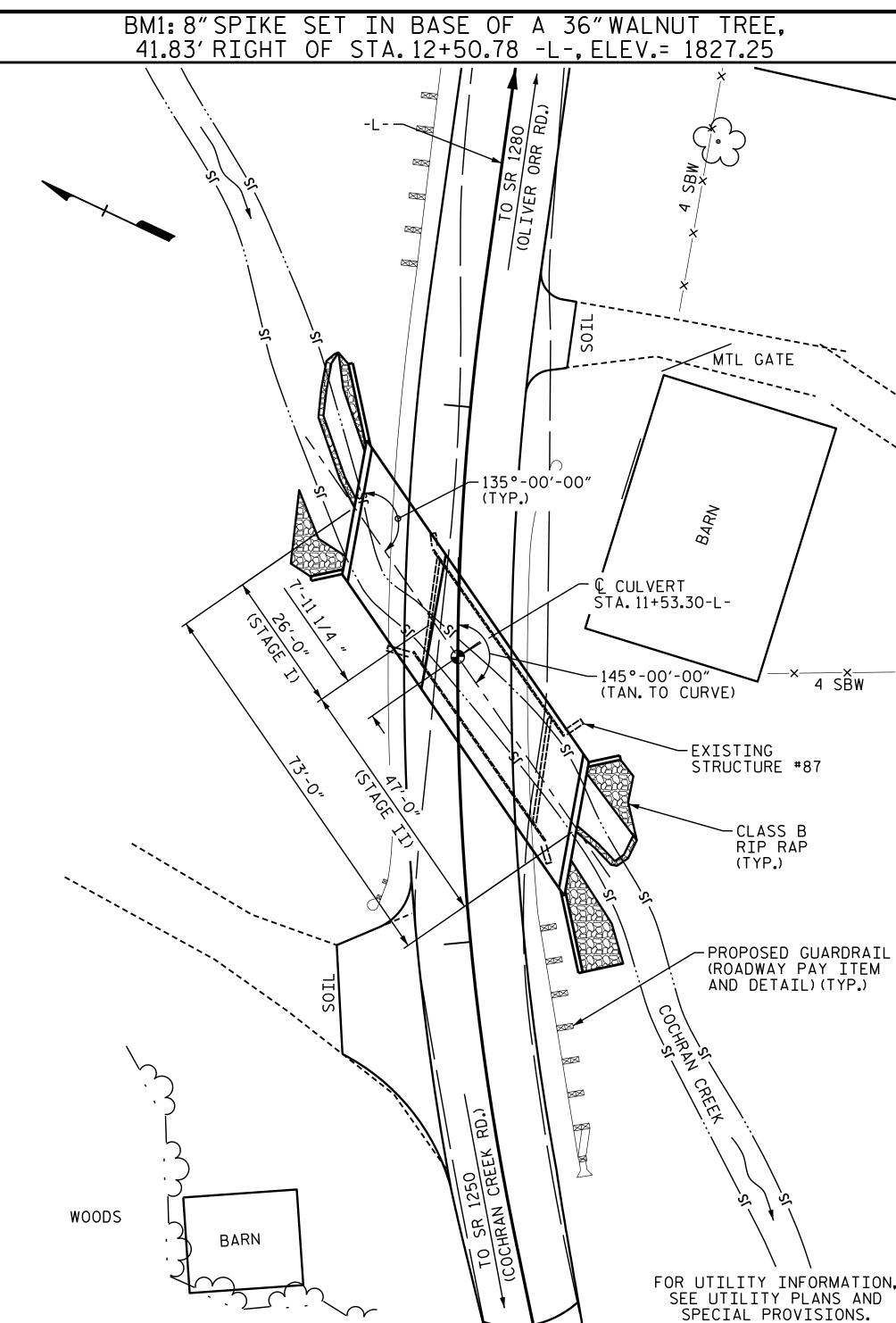
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NOTES

ASSUMED LIVE LOAD......HL-93 OR ALTERNATE LOADING.

MAXIMUM DESIGN FILL......1.3'

MINIMUM DESIGN FILL.....1.1'

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR CULVERT DIVERSION DETAILS, SEE EROSION CONTROL PLANS.

3" Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.

CONCRETE IN STAGE I OR STAGE II CULVERT TO BE POURED IN THE FOLLOWING ORDER:

1. WING FOOTINGS AND FLOOR SLAB FOR PHASE I, INCLUDING 4" OF ALL VERTICAL WALLS OF PHASE I.

2. THE REMAINING PORTIONS OF THE WALLS OF PHASE I AND WINGS FULL HEIGHT.

3. WING FOOTING AND FLOOR SLAB FOR PHASE II, INCLUDING  $4^{\prime\prime}$  OF ALL VERTICAL WALLS OF PHASE II.

4. THE REMAINING PORTIONS OF THE WALLS OF PHASE II AND WINGS FULL HEIGHT.

5. ROOF SLAB AND HEADWALLS OF PHASE II.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18- EVALUATING SCOUR AT BRIDGES."

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF A SINGLE SPAN: 20'-6"WITH AN ASPHALT WEARING SURFACE ON A TIMBER DECK AND TIMBER JOISTS HAVING A CLEAR ROADWAY WIDTH OF 24'-0"SUPPORTED ON A SUBSTRUCTURE OF TIMBER CAPS ON TIMBER POSTS, AND CONCRETE SILLS SHALL BE REMOVED. THE EXISTING STRUCTURE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED CULVERT, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL REINFORCING STEEL IN THE INTERIOR FACE OF THE EXTERIOR WALL ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH SHALL BE AS PROVIDED IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA WEIGHT OF STEEL DUE TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.

\*FOR LIMITS OF TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE TRAFFIC CONTROL PLANS. FOR PAY ITEM FOR TEMPORARY SHORING FOR MAINTENANCE OF TRAFFIC, SEE ROADWAY PLANS.

DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REINFORCING STEEL EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.

THE EXISTING BRIDGE SHALL BE PARTIALLY REMOVED AS SHOWN IN THE TRAFFIC CONTROL PLANS. THE EXISTING RAIL SHALL BE REMOVED AND REATTACHED TO THE REMAINING PORTION OF THE EXISTING BRIDGE. FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF THE CULVERT BEFORE STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY TAKE CARE OF THE FILL.

AT THE CONTRACTOR'S OPTION HE MAY SUBMIT, TO THE ENGINEER FOR APPROVAL, DESIGN AND DETAIL DRAWINGS FOR A PRECAST REINFORCED CONCRETE BOX CULVERT IN LIEU OF THE CAST-IN-PLACE CULVERT SHOWN ON THE PLANS. THE DESIGN SHALL PROVIDE THE SAME SIZE AND NUMBER OF BARRELS AS USED ON THE CAST-IN-PLACE DESIGN. FOR OPTIONAL PRECAST REINFORCED CONCRETE BOX CULVERT, SEE SPECIAL PROVISIONS.

TOTAL STRUCTURE	QUANTITIES
CLASS A CONCRETE  STAGE I  STAGE II  TOTAL	56.8 CU. YDS. 93.0 CU. YDS. 149.8 CU. YDS.
REINFORCING STEEL STAGE I STAGE II TOTAL	8,503 LBS. 13,953 LBS. 22,456 LBS.
CULVERT EXCAVATION	LUMP SUM
FOUNDATION COND.MAT'L.  STAGE I  STAGE II  TOTAL	33 TONS 60 TONS 93 TONS
REMOVAL OF EXISTING STRUCTURE	LUMP SUM
TEMPORARY EDGE BEAM	LUMP SUM
CLASS B RIP RAP	27 TONS
GEOTEXTILE FOR DRAINAGE	30 SQ. YDS.
ASBESTOS ASSESSMENT	LUMP SUM

## HYDRAULIC DATA

DESIGN DISCHARGE = 380 CFS
FREQUENCY OF DESIGN FLOOD = 5 YEARS
DESIGN HIGH WATER ELEVATION = 1821.6
DRAINAGE AREA = 2.2 SQ.MI.
BASE DISCHARGE (Q100) = 1000 CFS
BASE HIGH WATER ELEVATION = 1824.09

# OVERTOPPING DATA

OVERTOPPING DISCHARGE = 450 CFS FREQUENCY OF OVERTOPPING FLOOD = 5+ YEARS OVERTOPPING FLOOD ELEVATION = 1822.4

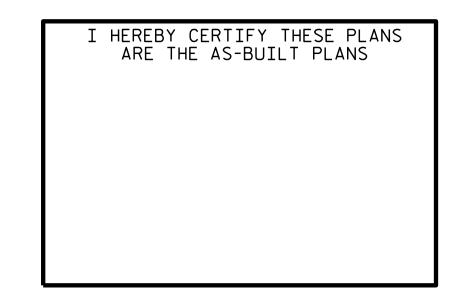
# HORIZONTAL CURVE DATA

PI STA 11+33.77 △ = 19°-44′-13.2″(RT) D = 11°-00′-00.0″ L = 179.43′ T = 90.61′ R = 520.87′

# GRADE DATA

GRADE POINT ELEV. @ STA. 11+53.30 -L-INVERT ELEV. @ STA. 11+53.30-L-ROADWAY SIDE SLOPES

= 1822.13 = 1814.99 = VARIES



SHEET 1 OF 11

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> SEAL 19299

NGINEER .

PROJECT NO. 17BP.14.R.79

GRAHAM COUNTY

STATION: 11+53.30 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

REPLACES BRIDGE #87

DOUBLE 8FT. X 5FT. CONCRETE BOX CULVERT 145° SKEW

REVISIONS

O. BY: DATE: NO. BY: DATE:

TOTAL SHEETS

11

EL. 1817.9 ± EL. 1822.5 ± EL. 1815.4 ± EL. 1817.9 ± EL. 1817.5 ± EL. 1816.3 ± EL.1814.5± EL. 1815.2 ± EL. 1818.4 ± EL. 1816.3 ± EL. 1815.4 ± 5′ 0″ 25' 0" 22′ 6″ 15' 0" 20′ 6″ 15′ 0″ 15′ 0″

PROFILE ALONG & CULVERT

WSP USA Inc.
434 FAYETTEVILLE STREET
SUITE 1500
RALEIGH, NC 27601
TEL: 1.919.836.4040

LICENSE NO. F-0165

DATE: .

DESIGN ENGINEER OF RECORD:

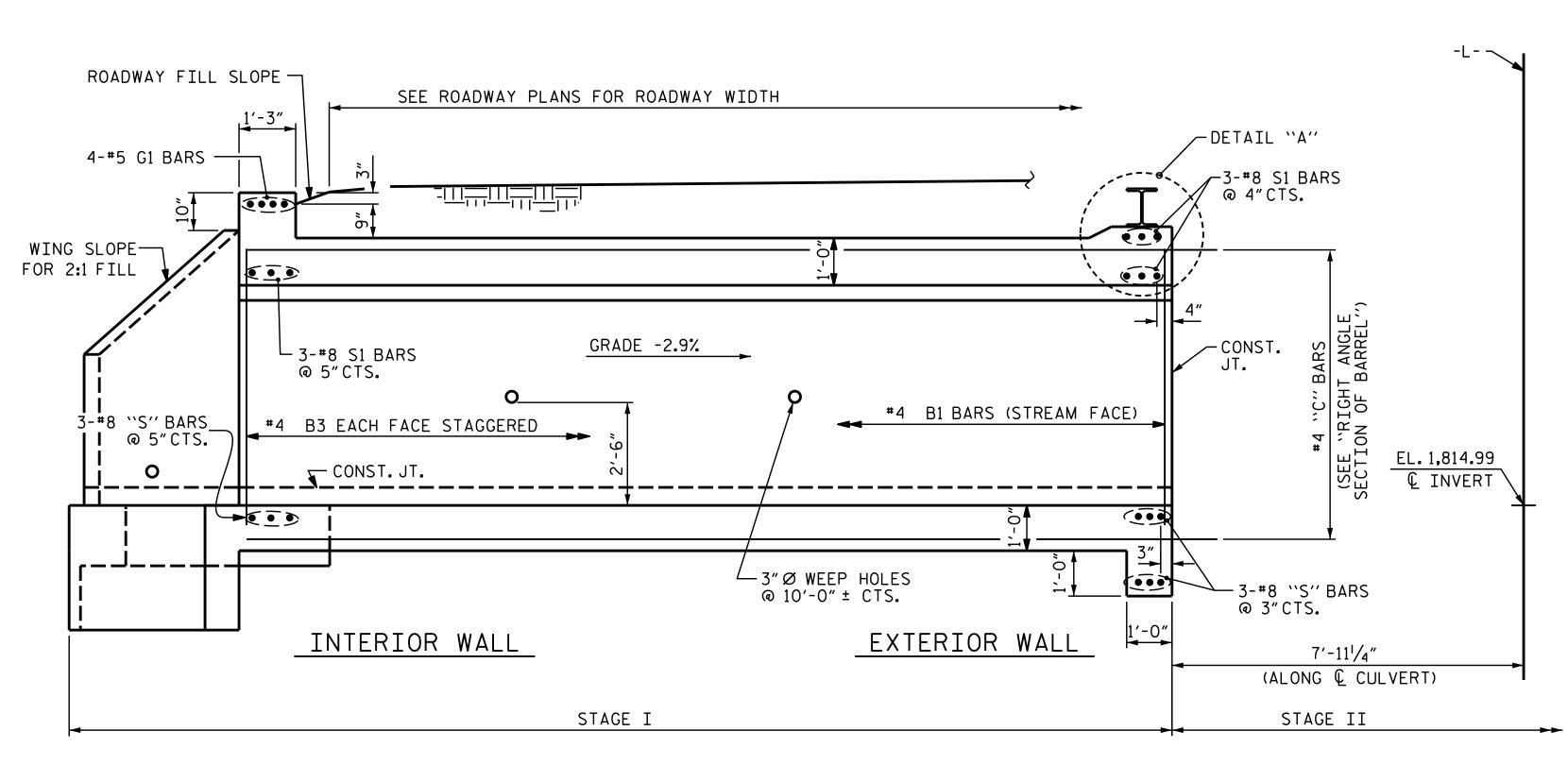
12/14/2018

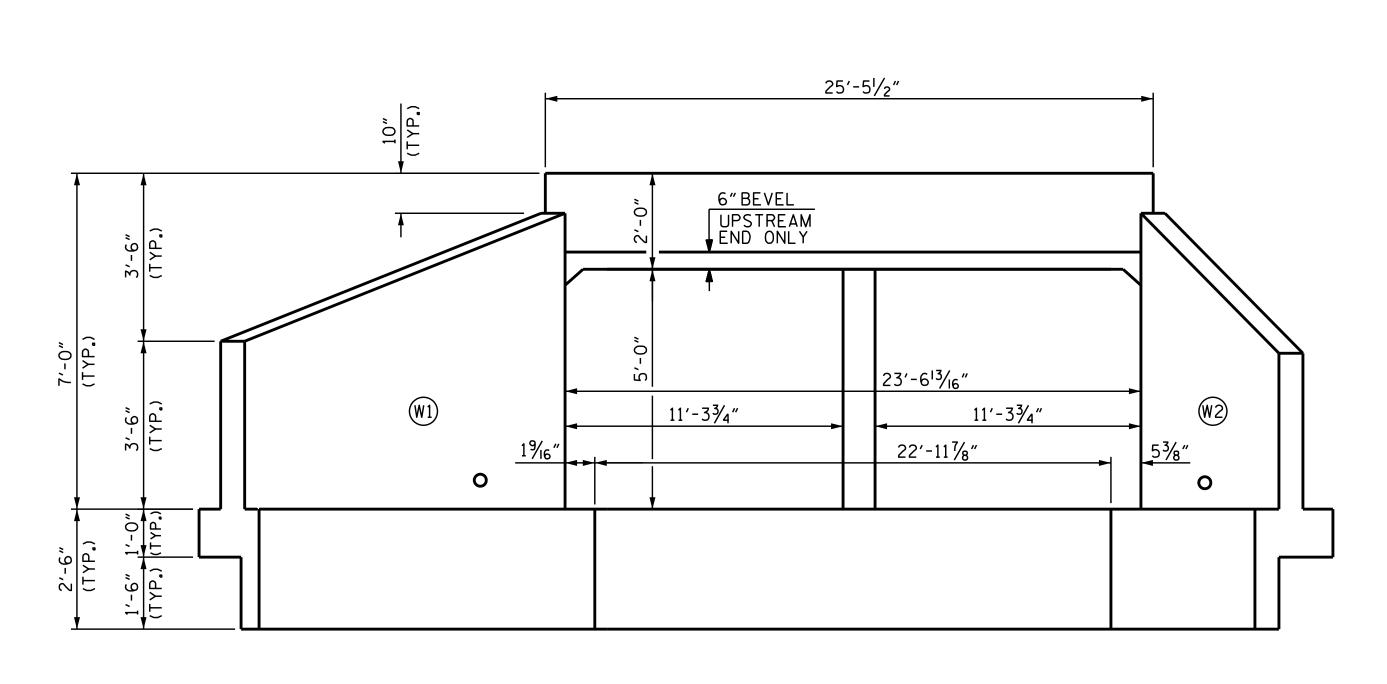
Thomas M. Harris

DRAWN BY: M. HOBBS DATE: 10/2014
CHECKED BY: M. MILLS DATE: 10/2014

SKETCH

LOCATION

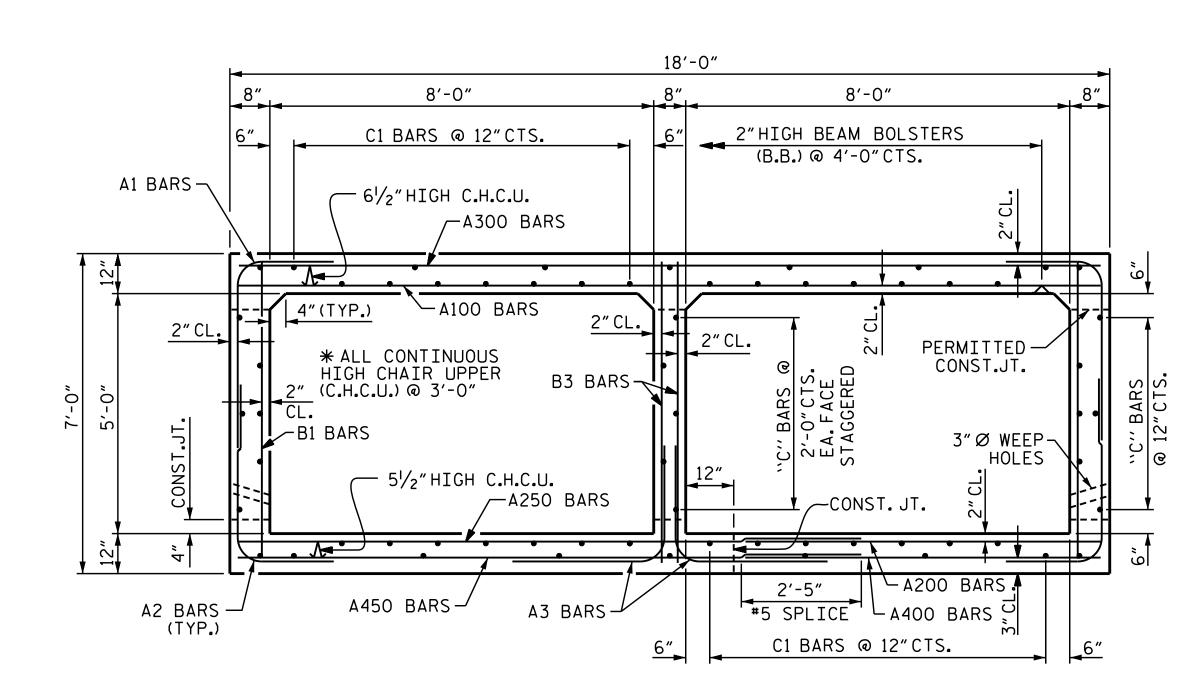




# CULVERT SECTION NORMAL TO ROADWAY (STAGE I)

# END ELEVATION NORMAL TO HEADWALL SKEW

(LOOKING DOWNSTREAM)





1'-4"

1'-4"

TEMPORARY
EDGE BEAM
WIOX45

STAGGERED @ 4"CTS. STARTING
6"± FROM BEAM ENDS
(SEE NOTE)

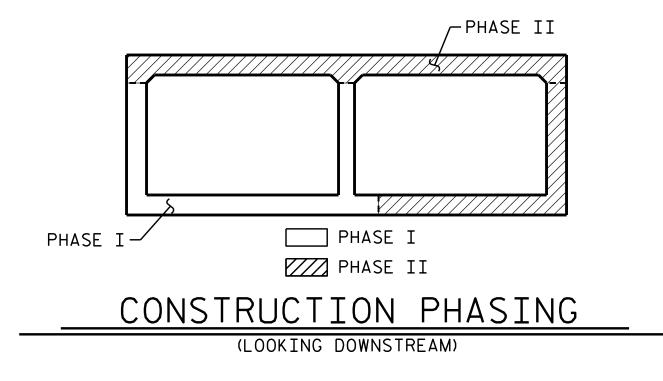
3-\*8 \$1 BARS
@ 4"CTS.

\*4 "C" BAR

\*4 "C" BAR

NOTE: AFTER STAGE II IS COMPLETE, THE BEAM SHALL BE REMOVED AND ANCHOR BOLTS CUT SMOOTH AND EPOXY APPLIED TO EXPOSED BOLTS TO PREVENT CORROSION.

DETAIL "A"



PROJECT NO. 17BP.14.R.79

GRAHAM COUNTY

STATION: 11+53.30 -L-

SHEET 2 OF 11

SE AL 19299 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

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CONCRETE BOX CULVERT STAGE I

REVISIONS

BY: DATE: NO. BY: DATE: C-2

3 TOTAL SHEETS
11

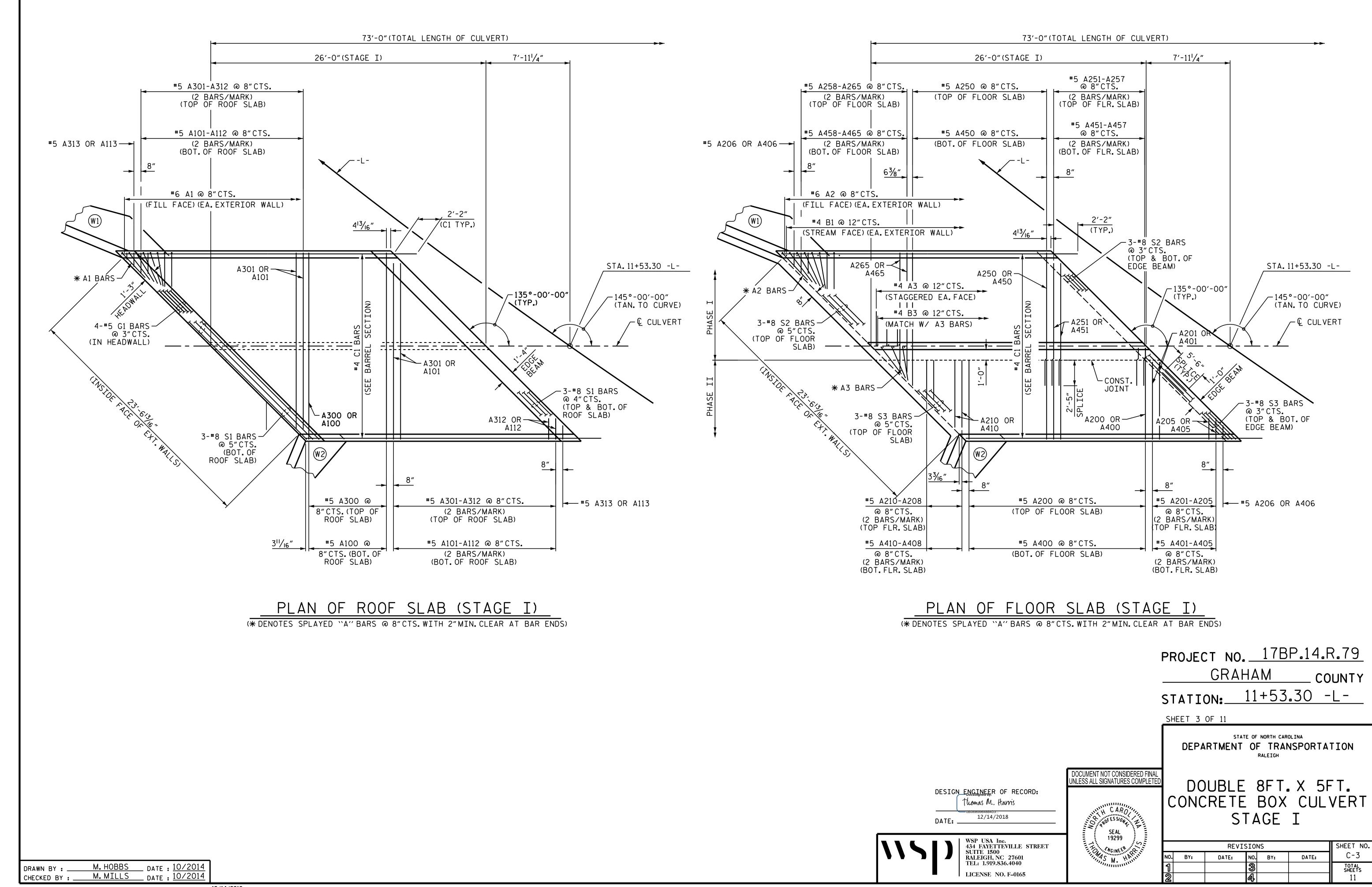
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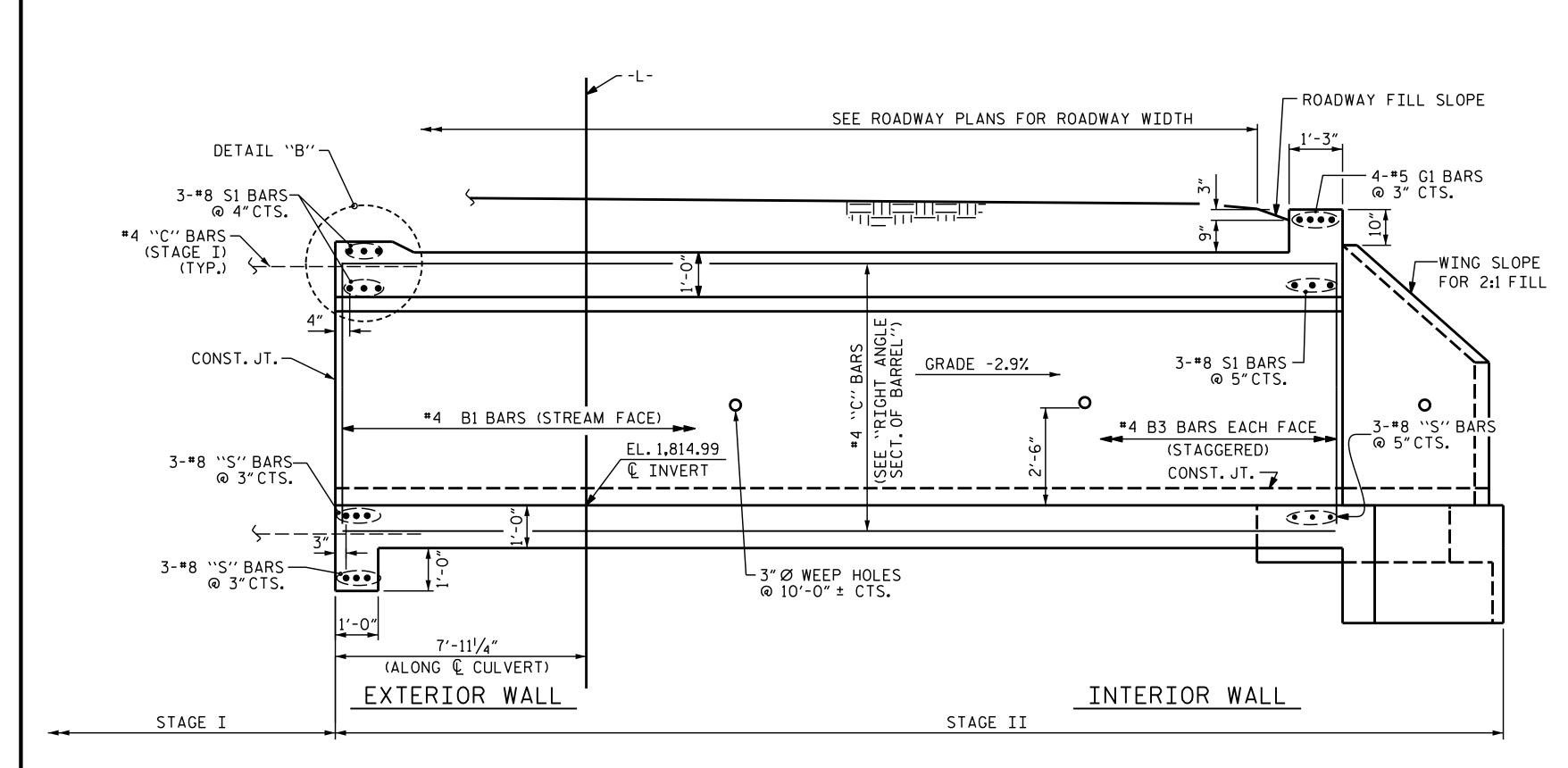
Thomas M. Harris

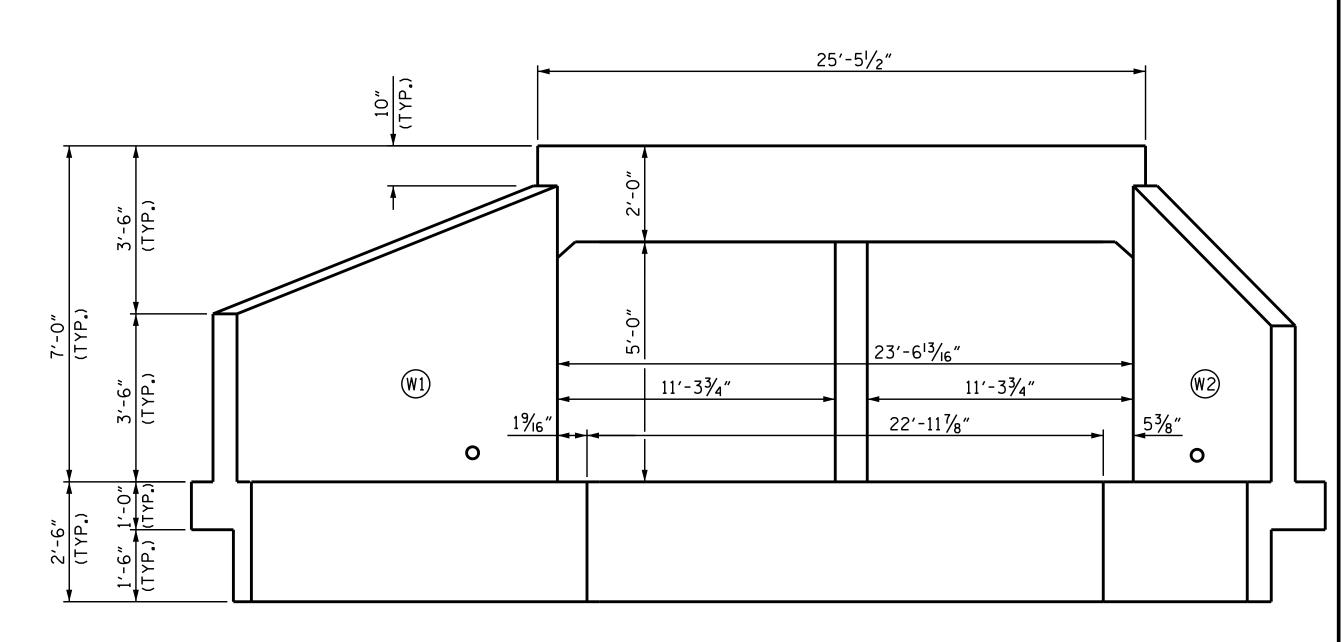
DATE: 12/14/2018

WSP USA Inc.
434 FAYETTEVILLE STREET
SUITE 1500
RALEIGH, NC 27601
TEL: 1.919.836.4040
LICENSE NO. F-0165

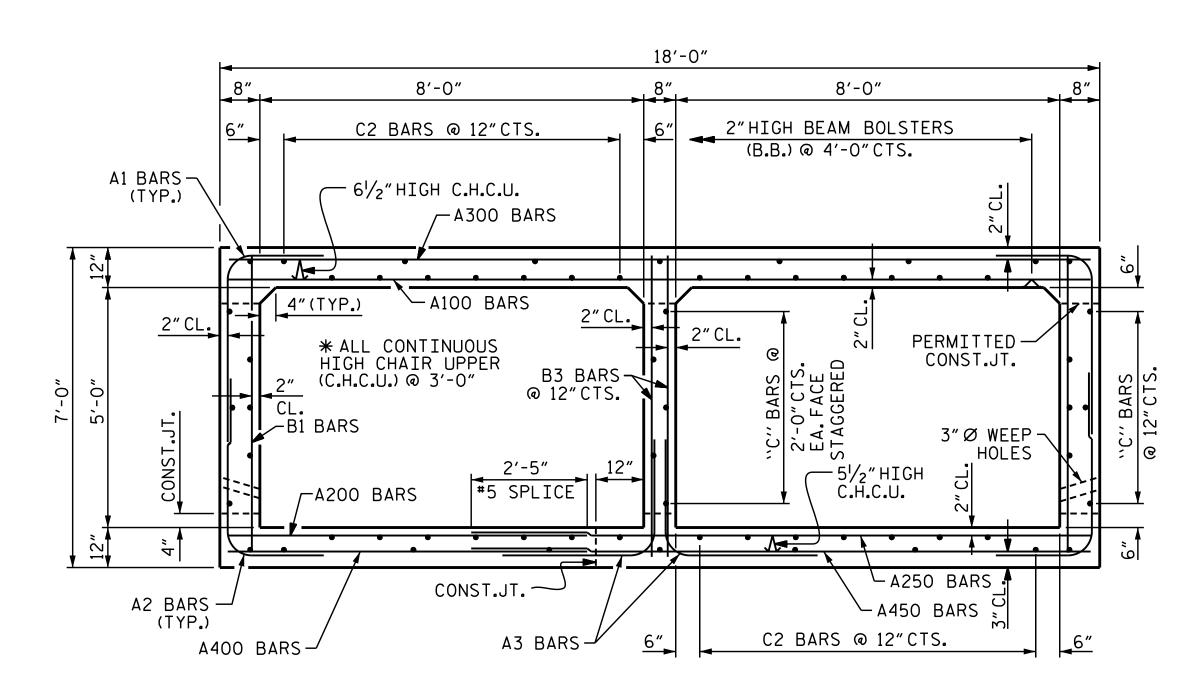
DRAWN BY: M. HOBBS DATE: 10/2014
CHECKED BY: M. MILLS DATE: 10/2014







# CULVERT SECTION NORMAL TO ROADWAY (STAGE II)



# RIGHT ANGLE SECTION OF BARREL

THERE ARE 63 "C" BARS IN SECTION OF BARREL.

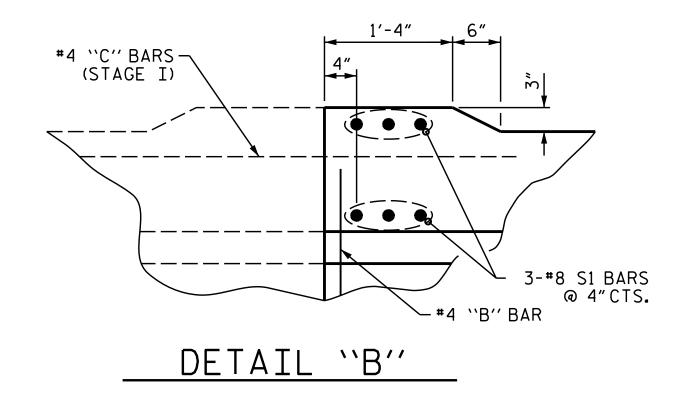
# END ELEVATION NORMAL TO HEADWALL SKEW

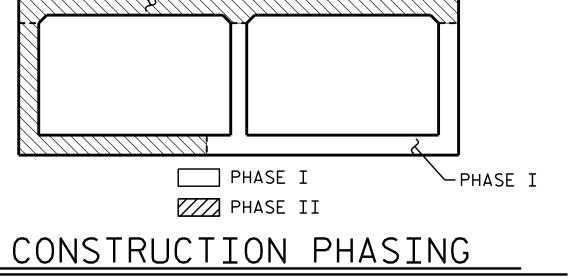
(LOOKING UPSTREAM)

PHASE II-

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SEAL 19299





(LOOKING DOWNSTREAM)

PROJECT NO. 17BP.14.R.79 GRAHAM COUNTY STATION: 11+53.30 -L-

SHEET 4 OF 11

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DOUBLE 8FT. X 5FT. CONCRETE BOX CULVERT (STAGE II)

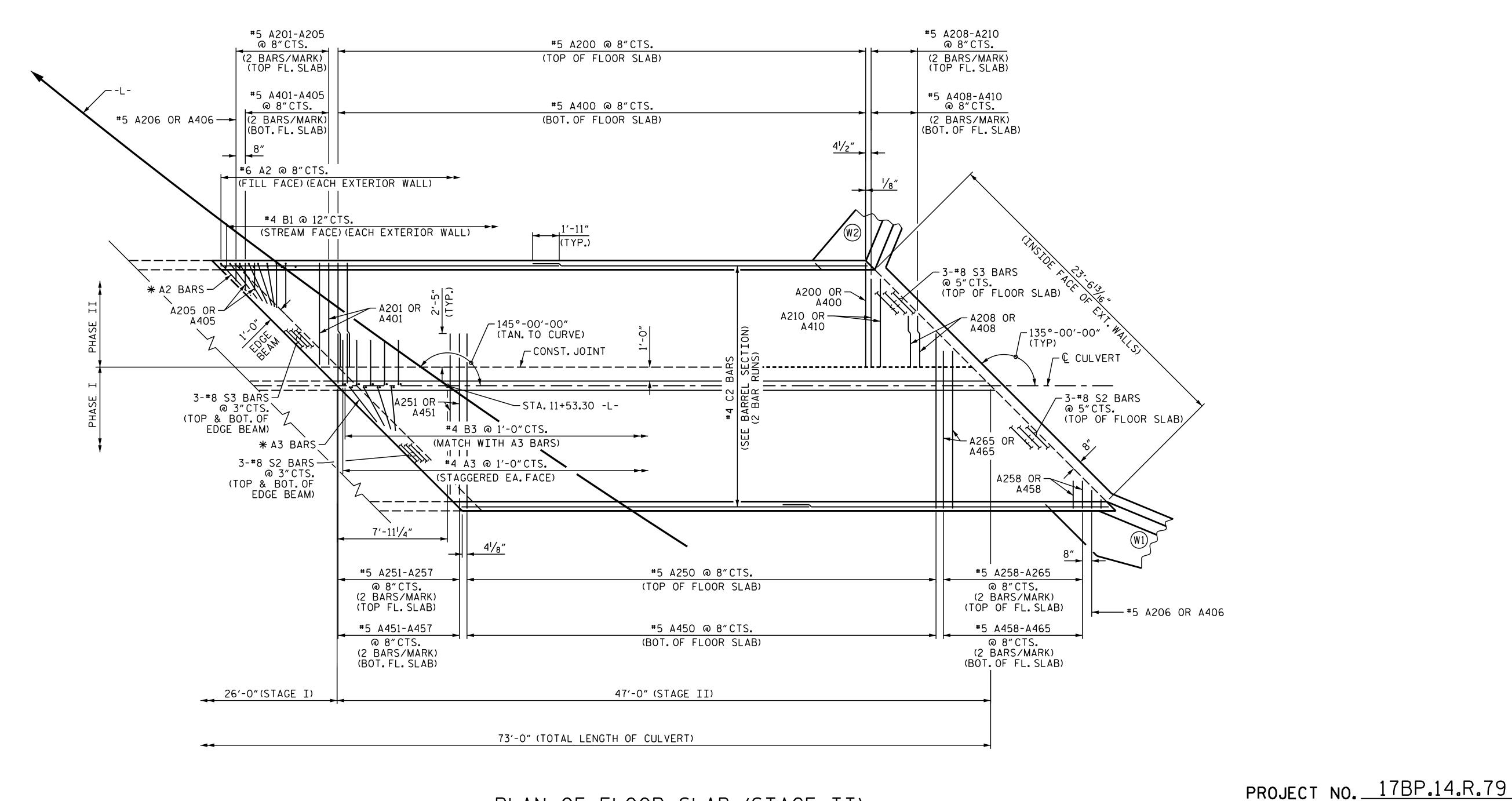
SHEET NO. REVISIONS NO. BY: C-4 DATE: BY: DATE: TOTAL SHEETS

DESIGN ENGINEER OF RECORD: Thomas M. Harris

12/14/2018 DATE: \_\_



M. HOBBS DATE: 10/2014
DATE: 10/2014 CHECKED BY : \_\_ 



# PLAN OF FLOOR SLAB (STAGE II)

(\* DENOTES SPLAYED "A" BARS @ 8"CTS. WITH 2"MIN. CLEAR AT BAR ENDS)

GRAHAM COUNTY
STATION: 11+53.30 -L-

SHEET 5 OF 11

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE

SEAL 19299 DEPARTMENT OF TRANSPORTATION
RALEIGH

DOUBLE 8FT.X 5FT.
CONCRETE BOX CULVERT
STAGE II

REVISIONS

BY: DATE: NO. BY: DATE: C-5

3 TOTAL SHEETS
11

DESIGN ENGINEER OF RECORD:

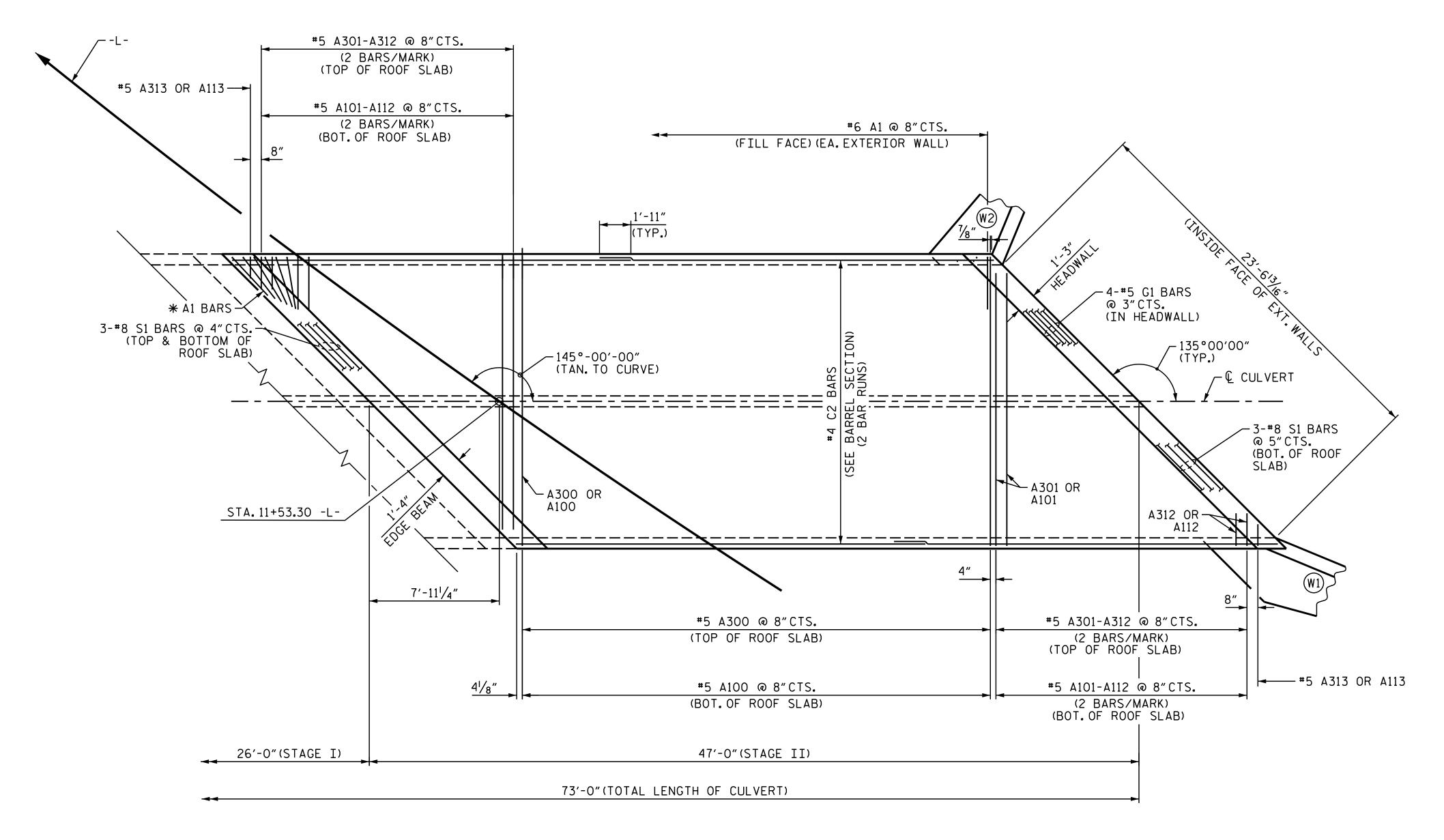
Thomas M. Harris

OB089699648B4D3...
12/14/2018

WSP USA Inc.
434 FAYETTEVILLE STREET

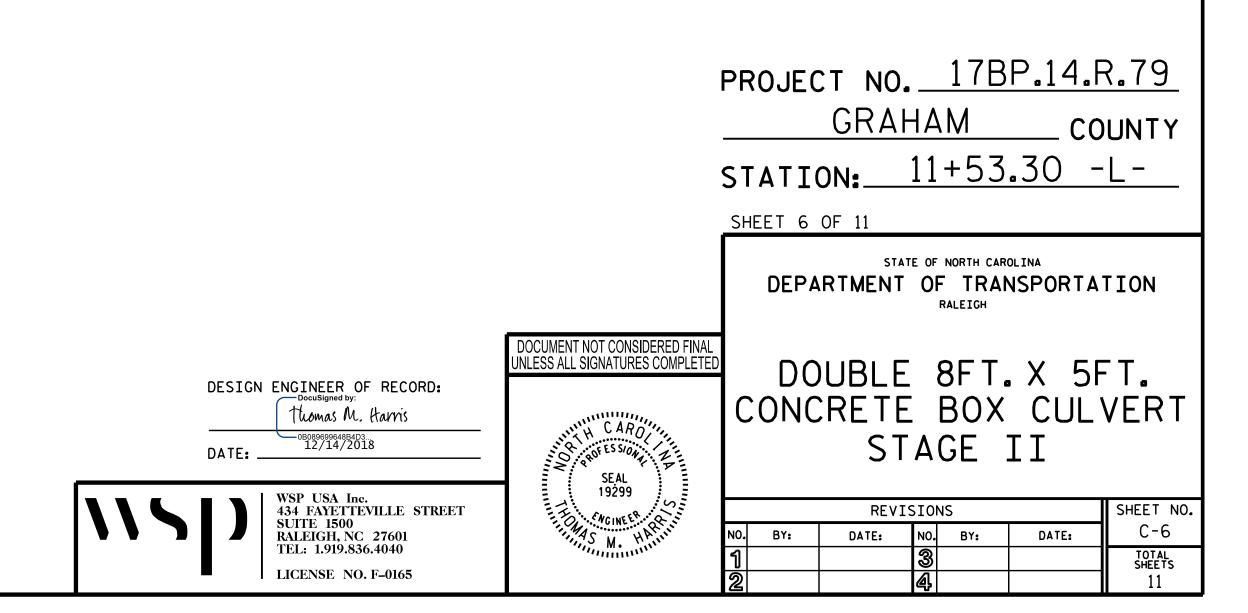
WSP USA Inc.
434 FAYETTEVILLE STREET
SUITE 1500
RALEIGH, NC 27601
TEL: 1.919.836.4040
LICENSE NO. F-0165

DRAWN BY : M. HOBBS DATE : 10/2014
CHECKED BY : M. MILLS DATE : 10/2014

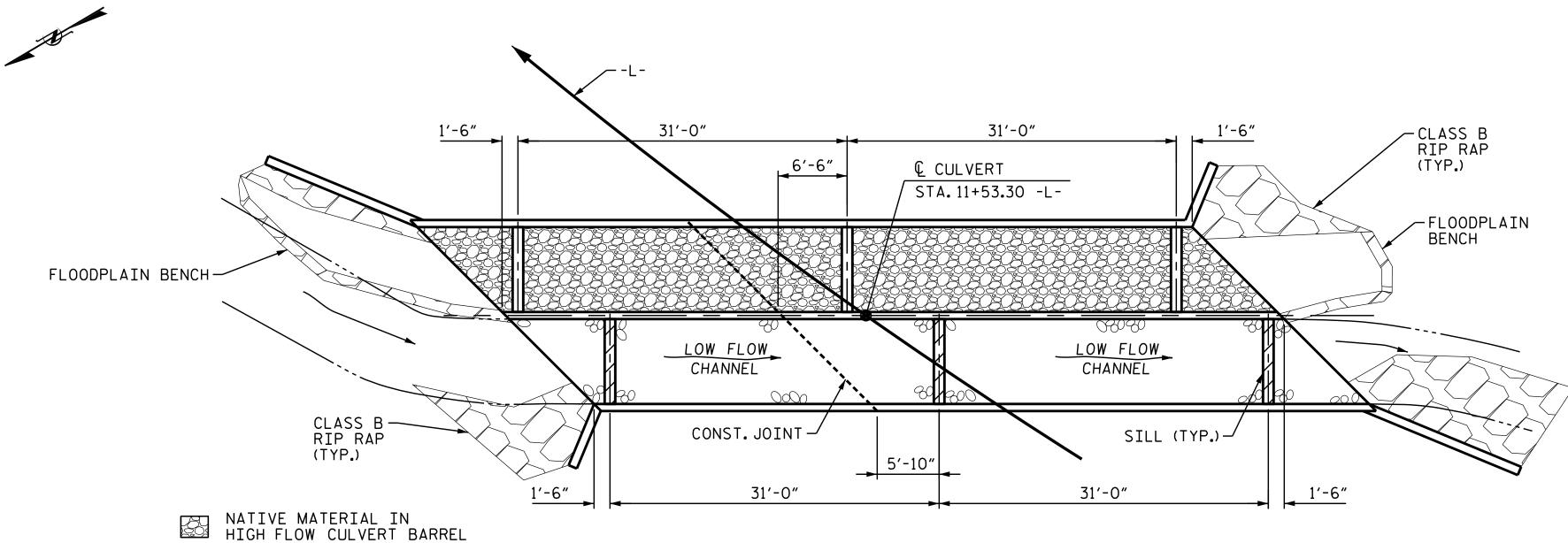


PLAN OF ROOF SLAB (STAGE II)

(\* DENOTES SPLAYED "A" BARS @ 8"CTS. WITH 2"MIN. CLEAR AT BAR ENDS)



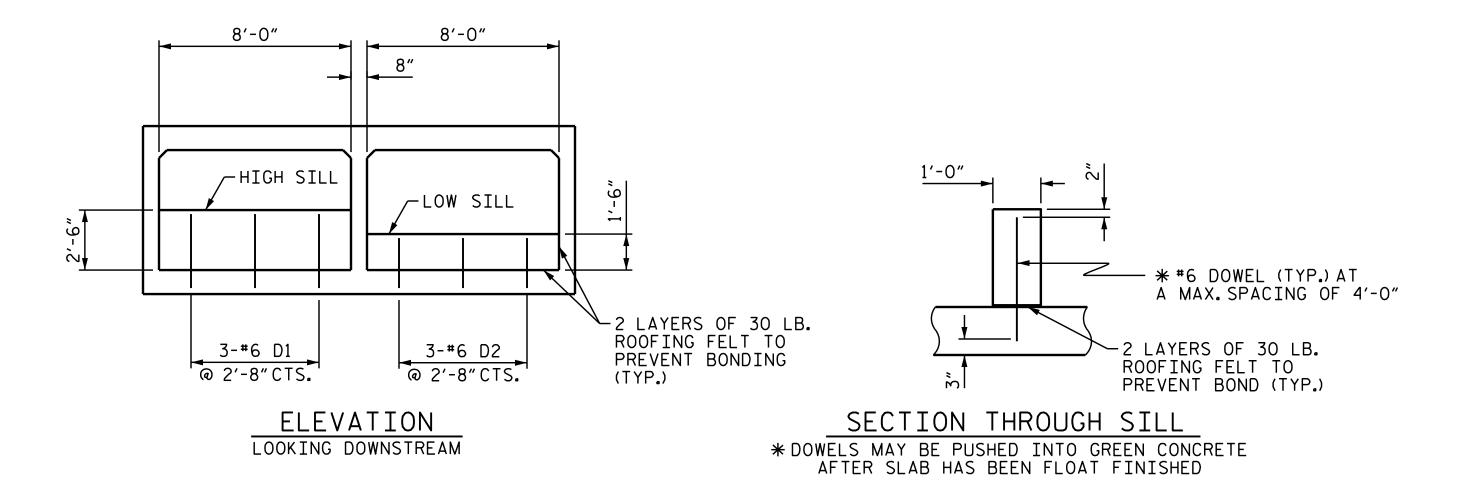
DRAWN BY : M. HOBBS DATE : 10/2014
CHECKED BY : M. MILLS DATE : 10/2014



NATIVE MATERIAL IN LOW FLOW CULVERT BARREL

PLAN OF FLOOR SILL LAYOUT

LOW FLOW SILLS DENOTED BY CROSSHATCHED AREA.



CULVERT SILL DETAILS

DESIGN ENGINEER OF RECORD:

Thomas M. Harris

0808969964884D3...
12/14/2018

DATE: 12/14/2018



#### NOTES

NATIVE MATERIAL EXCAVATED FROM THE EXISTING STREAM BED OR FLOOD PLAIN AT THE PROJECT SITE DURING CULVERT CONSTRUCTION SHALL BE STOCKPILED AND LATER PLACED IN THE PROPOSED CULVERT BETWEEN SILLS TO PROVIDE A CONTINUOUS LOW FLOW CHANNEL. NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS.

THE STOCKPILED NATIVE MATERIAL SHALL BE PLACED AS SHOWN IN THE "PLAN OF FLOOR SILL LAYOUT" SKETCH TO PROVIDE A 1'-6" DEPTH LOW FLOW CHANNEL BETWEEN LOW FLOW SILLS, AND SHALL BE PLACED TO THE DEPTH OF 2'-6" BETWEEN HIGH FLOW SILLS.

IF NECESSARY, SUPPLEMENTAL STONE OF SIMILAR CHARACTERISTIC OF THE NATIVE MATERIAL MAYBE USED WITH APPROVAL BY ENGINEER. SUPPLEMENTAL STONE WILL BE PAID FOR UNDER THE CONTRACT PRICE FOR CHANNEL SUBSTRATE MATERIAL FOR NATIVE STONE.

THE ENTIRE COST OF WORK REQUIRED TO PLACE THE EXCAVATED MATERIAL SHALL BE INCLUDED IN THE CONTRACT LUMP SUM PRICE BID FOR CULVERT EXCAVATION.

THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS. NO SEPARATE PAYMENT WILL BE MADE FOR THE 30 LB. ROOFING FELT.

TOP OF LOW FLOW SILLS SHOULD MATCH STREAM BED ELEVATION IN LOW FLOW CHANNEL OF STREAM. (THALWEG)

DO NOT SET ELEVATION OF HIGH SILL ABOVE BANK FULL.

PROJECT NO. 17BP.14.R.79

GRAHAM COUNTY

STATION: 11+53.30 -L-

SHEET 7 OF 11

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SEAL 19299 DEPARTMENT OF TRANSPORTATION
RALEIGH

DOUBLE 8FT.X 5FT.
CONCRETE BOX CULVERT
STAGES I & II
BAFFLE/SILL LAYOUT

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	C-7
		3			TOTAL SHEETS
		4			11

DRAWN BY : M. HOBBS DATE : 10/2014
CHECKED BY : M. MILLS DATE : 10/2014

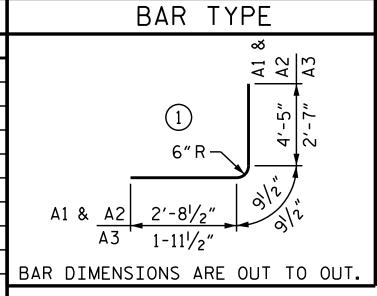
FBE3A883ED	.3A883ED																						
									————— BAR SCHEDULE STAGE II ————														
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
A1	78	#6	1	7'-11"	927	A400	26	#5	STR	7'-6"	203	A1	142	#6	1	7'-11"	1689	A400	58	#5	STR	7′-6″	454
A2	78	#6	1	7'-11"	927	A401	2	#5	STR	7′-4″	15	A2	142	#6	1	7′-11″	1689	A401	2	#5	STR	7'-4"	15
А3	52	#4	1	5'-4"	185	A402	2	#5	STR	6′-0″	13	А3	94	#4	1	5′-4″	335	A402	2	#5	STR	6'-0"	13
						A403	2	#5	STR	4′-8″	10							A403	2	#5	STR	4'-8"	10
A100	12	#5	STR	17'-8"	221	Δ404	2	#5	STR	3'-4"	7	A100	44	#5	STR	17′-8″	811	A404	2	#5	STR	3'-4"	7
A101	4	#5	STR	16'-8"	70	A405	2	#5	STR	2'-0"	4	A101	4	#5	STR	16′-8″	70	A405	2	#5	STR	2'-0"	4
A102	4	#5	STR	15'-4"	64	A406	2	#5	STR	1'-4"	3	A102	4	#5	STR	15'-4"	64	A406	2	#5	STR	1'-4"	3
A103	4	#5	STR	14'-0"	58							A103	4	#5	STR	14'-0"	58						
A104	4	#5	STR	12'-8"	53	A408	2	#5	STR	3′-8″	8	A104	4	#5	STR	12'-8"	53	A408	2	#5	STR	3′-8″	8
A105	4	#5	STR	11'-4"	47	A409	2	#5	STR	5′-0″	10	A105	4	#5	STR	11'-4"	47	A409	2	#5	STR	5′-0″	10
A106	4	#5	STR	10'-0"	42	A410	2	#5	STR	6′-4″	13	A106	4	#5	STR	10'-0"	42	A410	2	#5	STR	6′-4″	13
A107	4	#5	STR	8'-8"	36					10. 7.		A107	4	#5	STR	8'-8"	36				0.75	40: 7"	
A108	4	#5	STR	7'-4"	31	A450	20	#5	STR	12'-7"	262	A108	4	#5	STR	7'-4"	31	A450	52	#5	STR	12'-7"	682
A110	4	#5	STR	6'-0"	25	A451	2	#5	STR	11'-7"	24	A109	4	#5	STR	6'-0"	25	A451	2	#5	STR	11'-7"	24
A110	4	#5 #5	STR STR	4'-8" 3'-4"	19	A452 A453	2	#5	STR STR	10′-3″	21	A110	4	#5	STR STR	4'-8" 3'-4"	19 14	A452	2	#5	STR STR	10'-3" 8'-11"	21
A111 A112	4	#5	STR	2'-0"	14	A453	2	#5 #5	STR	8'-11" 7'-7"	19 16	A111 A112	4	#5 #5	STR	2'-0"	8	A453 A454	2	#5 #5	STR	7'-7"	19 16
A112	2	#5	STR	1'-4"	7	A454	2	#5	STR	6'-3"	13	A112	2	#5	STR	1'-4"	3	A455	2	#5	STR	6'-3"	13
4112		+ -	7 111	1 7		A455	2	#5	STR	4'-11"	10	7117			5111	1 7		A455	2	#5	STR	4'-11"	10
A200	26	#5	STR	7′-6″	203	A457	2	#5	STR	3'-7"	7	A200	58	#5	STR	7′-6″	454	A457	2	#5	STR	3'-7"	7
A201	2	#5	STR	7'-4"	15	A458	2	#5	STR	2'-0"	4	A201	2	#5	STR	7'-4"	15	A458	2	#5	STR	2'-0"	4
A202	2	#5	STR	6'-0"	13	A459	2	#5	STR	3'-4"	7	A202	2	#5	STR	6′-0″	13	A459	2	#5	STR	3'-4"	7
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A205	2	#5	STR	2'-0"	4	A462	2	#5	STR	7′-4″	15	A205	2	#5	STR	2'-0"	4	A462	2	#5	STR	7′-4″	15
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						A464	2	#5	STR	10'-0"	21							A464	2	#5	STR	10'-0"	21
A208	2	#5	STR	3′-8″	8	A465	2	#5	STR	11'-4"	24	A208	2	#5	STR	3′-8″	8	A465	2	#5	STR	11'-4"	24
A209	2	#5	STR	5'-0"	10							A209	2	#5	STR	5′-0″	10						
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4250	20	#.	CTD	10/ 7//	262	B3	52	#4	STR	6′-6″	226	4050	<u> </u>	#.	CTD	10/ 7//	600	B3	94	#4	STR	6′-6″	408
A250	20	#5	STR	12'-7"	262	C1	C 7	# 4	CTD	27/ 11//	1175	A250	52	#5	STR	12'-7"	682	CO	126	#4	CTD	24/ 7/	2041
A251 A252	2	#5 #5	STR STR	11'-7" 10'-3"	24	CI	63	#4	STR	27'-11"	1175	A251 A252	2	#5 #5	STR STR	11'-7" 10'-3"	24	C2	126	4	STR	24'-3"	2041
A252	2	#5	STR	8'-11"	19	D1	3	#6	STR	3'-1"	14	A253	2	#5	STR	8'-11"	19	D1	6	#6	STR	3′-1″	28
A254	2	#5	STR	7'-7"	16	D2	3	#6	STR	2'-1"	9	A254	2	#5	STR	7'-7"	16	D2	6	#6	STR	2'-1"	19
A255	2	#5	STR	6'-3"	13	52			3111			A255	2	#5	STR	6′-3″	13	<u> </u>			3111		
A256	2	#5	STR	4'-11"	10	G1	4	#5	STR	24'-10"	104	A256	2	#5	STR	4'-11"	10	G1	4	#5	STR	24′-10″	104
A257	2	#5	STR	3'-7"	7							A257	2	#5	STR	3'-7"	7						
A258	2	#5	STR	2'-0"	4	S1	9	#8	STR	24'-10"	597	A258	2	#5	STR	2'-0"	4	S1	9	#8	STR	24′-10″	597
A259	2	#5	STR	3'-4"	7	S2	9	#8	STR	19'-10"	477	A259	2	#5	STR	3'-4"	7	S2	9	#8	STR	19'-10"	477
A260	2	#5	STR	4′-8″	10	S3	9	#8	STR	10'-7"	254	A260	2	#5	STR	4′-8″	10	S3	9	#8	STR	10'-7"	254
A261	2	#5	STR	6′-0″	13							A261	2	#5	STR	6′-0″	13						
A262	2	#5	STR	7'-4"	15							A262	2	#5	STR	7'-4"	15						
A263	2	#5	STR	8'-8"	18	<u> </u>						A263	2	#5	STR	8'-8"	18						
A264	2	#5	STR	10'-0"	21							A264	2	#5	STR	10'-0"	21						<b></b>
A265	2	#5	STR	11'-4"	24							A265	2	#5	STR	11'-4"	24						<del>                                     </del>
A300	12	#5	STR	17'-8"	221							A300	44	#5	STR	17′-8″	811				-		<del>                                     </del>
A300	12	#5	STR	16'-8"	221 70							A300	44	#5	STR	16'-8"	70						+
A301	<u> </u>	#5	STR	15'-4"	64							A301	4	#5	STR	15'-4"	64				<del>                                     </del>		+
A302	4	#5	STR	14'-0"	58	<del> </del>						A302	4	#5	STR	14'-0"	58						
A304	4	#5	STR	12'-8"	53							A304	4	#5	STR	12'-8"	53						
A305	4	#5	STR	11'-4"	47							A305	4	#5	STR	11'-4"	47						
A306	4	#5	STR	10'-0"	42							A306	4	#5	STR	10'-0"	42						
A307	4	#5	STR	8'-8"	36							A307	4	#5	STR	8'-8"	36						
A308	4	#5	STR	7'-4"	31							A308	4	#5	STR	7′-4″	31						
A309	4	#5	STR	6′-0"	25							A309	4	#5	STR	6′-0"	25						
A310	4	#5	STR	4'-8"	19							A310	4	#5	STR	4′-8″	19						
A311	4	#5	STR	3'-4"	14							A311	4	#5	STR	3′-4″	14						
A312	4	#5	STR	2'-0"	8							A312	4	#5	STR	2'-0"	8						
A313	2	#5	STR	1'-4"	3							A313	2	#5	STR	1'-4"	3						
		<u> </u>																					
		<u> </u>				<u></u>								<u> </u>									
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REINFORCING STEEL

LBS. 8,043

REINFORCING STEEL

LBS. 13,493



LENGTHS	
UNCOATED	
1'-11"	
2′-5″	
3'-1"	
4'-3"	
5′-6″	
	UNCOATED  1'-11"  2'-5"  3'-1"  4'-3"

STAGE I QUANTITIES		
CLASS A CONCRETE  BARREL @1.704 CY/FT  WINGS, ETC	44.3	CY
SILLS TOTAL	1.2 56.8	_
REINFORCING STEEL		
BARREL & SILLS	8,043	_ LBS
WINGS, ETC	460	_ LBS
TOTAL	8,503	_ LBS

STAGE II QUANTITIES		
CLASS A CONCRETE  BARREL @1.704 CY/FT  WINGS, ETC  SILLS  TOTAL	80.1 11.3 1.6 93.0	_ CY _ CY
REINFORCING STEEL BARREL & SILLSWINGS,ETC TOTAL		LBS. LBS. LBS.

PROJECT NO. 17BP.14.R.79

GRAHAM COUNTY

STATION: 11+53.30 -L-

SHEET 8 OF 11

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SEAL 19299 STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

DOUBLE 8FT. X 5FT.
CONCRETE BOX CULVERT
STAGES I & II
BILL OF MATERIAL

REVISIONS

BY: DATE: NO. BY: DATE:

C-8

TOTAL SHEETS
11

DESIGN ENGINEER OF RECORD:

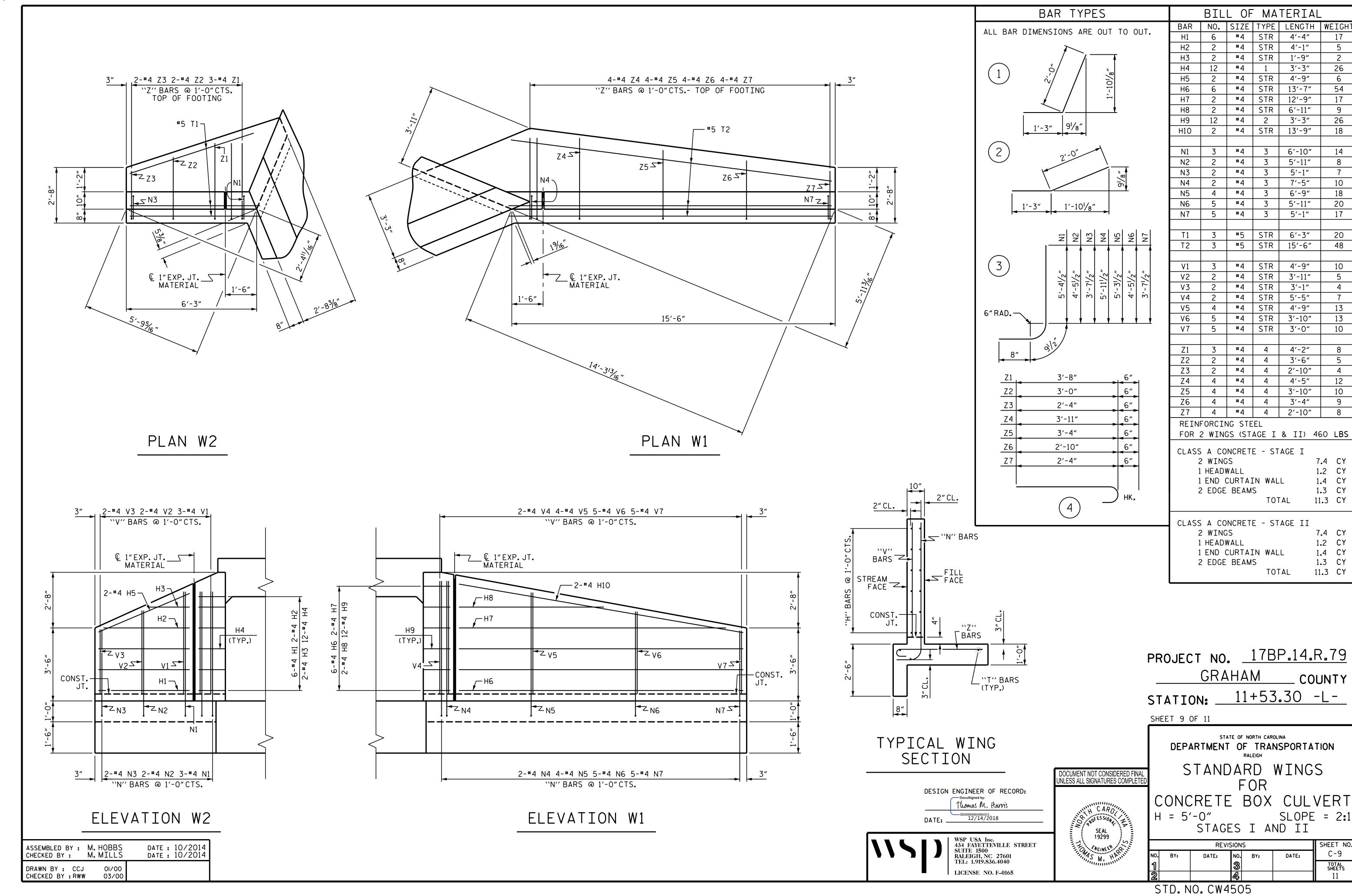
Thomas M. Harris

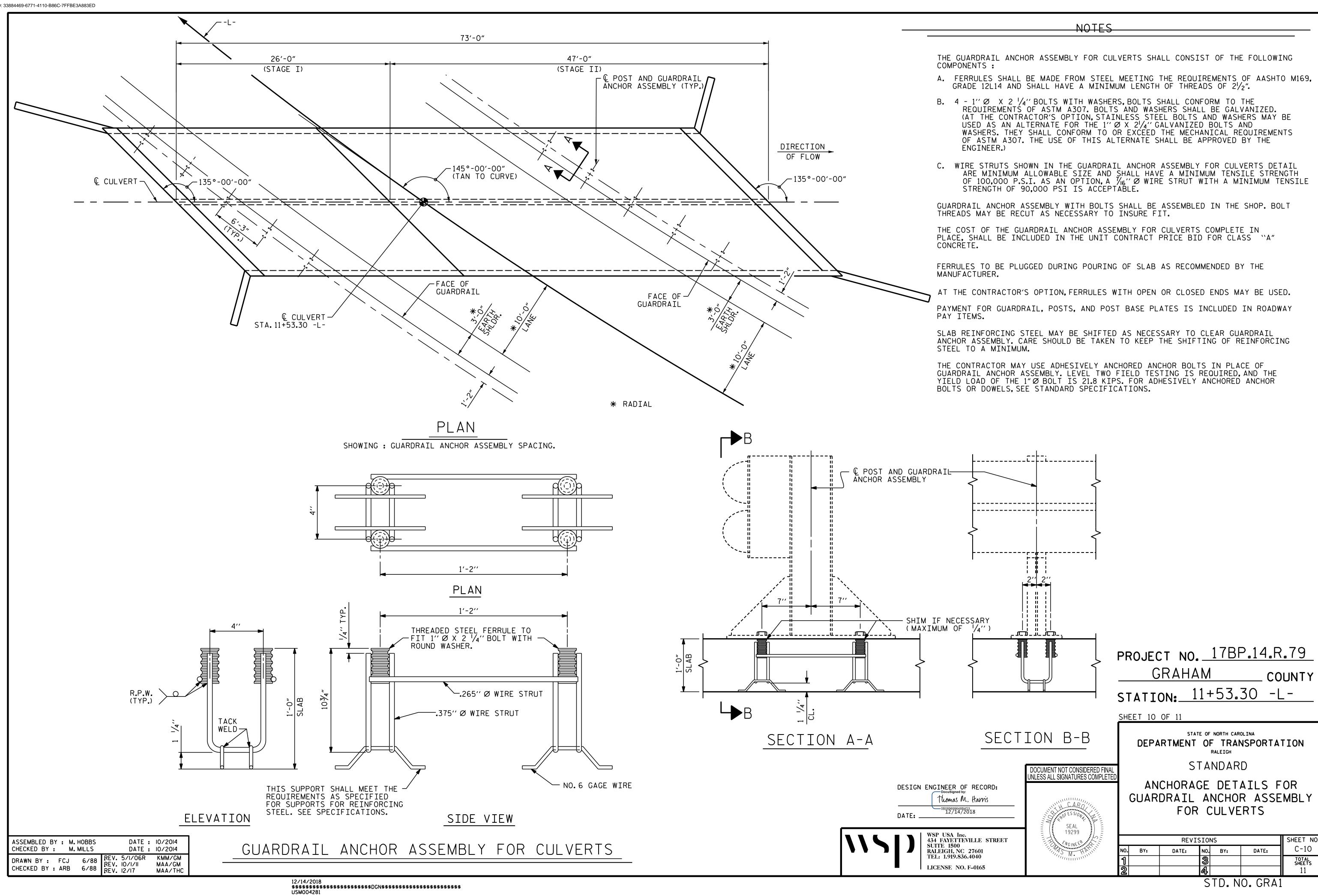
OB089699648B4D3...

12/14/2018



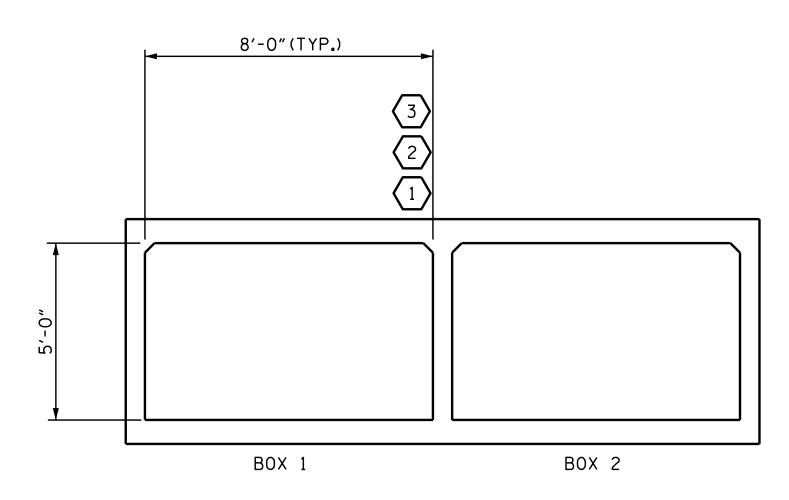
DRAWN BY: \_\_\_\_\_\_ M. HOBBS \_\_\_\_\_ DATE: 10/2014 CHECKED BY: \_\_\_\_\_ M. MILLS \_\_\_\_\_ DATE: 10/2014





# LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS

STRENGTH I LIMIT STATE																
										MOMENT				SHEAR		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y <sub>LL</sub> )	RATING FACTOR	BOX NO.	ELEMENT	DISTANCE FROM LEFT END OF ELEMENT (ft)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.00		1 <b>.</b> 75	1.27	1	TOP ·SLAB	4.67	1.00	1	TOP ·SLAB	8.67	•
DESIGN LOAD		HL-93 (OPERATING)	N/A	•	1.29		1.35	1.65	1	TOP ·SLAB	4.67	1.29	1	TOP ·SLAB	8.67	•
RATING		HS-20 (INVENTORY)	36.000	2	1.26	45:36	1.75	1.27	1	TOP ·SLAB	4.67	1.26	1	TOP ·SLAB	8.67	•
	_	HS-20 (OPERATING)	36.000	•	1.63	58:68	1.35	1.65	1	TOP ·SLAB	4.67	1.63	1	TOP ·SLAB	8.67	•
		SNSH	13 <b>.</b> 500	•	2 <b>.</b> 31	31:19	1.40	2 <b>.</b> 31	1	TOP ·SLAB	4.67	2.76	1	TOP ·SLAB	8.67	•
	Ш	SNGARBS2	20.000	•	2.16	43:20	1.40	2 <b>.1</b> 6	1	TOP ·SLAB	4.67	2 <b>.</b> 51	1	TOP ·SLAB	8.67	•
	ICL	SNAGRIS2	22.000	•	2 <b>.</b> 31	50:82	1.40	2 <b>.</b> 31	1	TOP ·SLAB	4.67	2 <b>.</b> 76	1	TOP ·SLAB	8.67	•
	VEHICLI (V)	SNCOTTS3	27.250	3	1.25	34:06	1.40	1 <b>.</b> 92	1	TOP ·SLAB	4.67	1.25	1	TOP ·SLAB	8.67	•
	$\square$ $\square$	SNAGGRS4	34 <b>.</b> 925	•	1.67	58:32	1.40	1.67	1	BOTTOM SLAB	8.67	1.77	1	TOP ·SLAB	8.67	•
	SINGL	SNS5A	35 <b>.</b> 550	•	1.58	56 <b>:</b> 17	1.40	1.85	1	BOTTOM SLAB	8 <b>.</b> 67	1.58	1	TOP ·SLAB	8.67	•
		SNS6A	39 <b>.</b> 950	•	1.58	63 <b>:</b> 12	1.40	1.72	1	BOTTOM SLAB	8 <b>.</b> 67	1.58	1	TOP ·SLAB	8.67	•
LEGAL LOAD		SNS7B	42.000	•	1.58	66:36	1.40	1.72	1	BOTTOM SLAB	8 <b>.</b> 67	1.58	1	TOP ·SLAB	8.67	•
RATING	ER	TNAGRIT3	33.000	•	2.25	74:25	1.40	2.25	1	TOP ·SLAB	8 <b>.</b> 67	2.60	1	TOP ·SLAB	8.67	•
	TRAIL	TNT4A	33.075	•	1.61	53:25	1.40	2.07	1	BOTTOM SLAB	8 <b>.</b> 67	1.61	1	TOP ·SLAB	8.67	•
	1 1	TNT6A	41.600	•	1.58	65:73	1.40	1.86	1	BOTTOM SLAB	8 <b>.</b> 67	1.58	1	TOP ·SLAB	8.67	•
	SEMI.	TNT7A	42.000	•	1.61	67:62	1.40	1.82	1	BOTTOM SLAB	8 <b>.</b> 67	1.61	1	TOP ·SLAB	8.67	•
	CTOR (TT)	TNT7B	42.000	•	1 <b>.</b> 58	66:36	1.40	1.93	1	TOP ·SLAB	8.67	1.58	1	TOP ·SLAB	8.67	•
	TRAC	TNAGRIT4	43.000	•	1.59	68:37	1.40	1.77	1	BOTTOM SLAB	8.67	1.59	1	TOP ·SLAB	8 <b>.</b> 67	•
	TRUCK	TNAGT5A	45.000	•	1.61	72:45	1.40	1.73	1	BOTTO₩ SLAB	8.67	1.61	1	TOP ·SLAB	8.67	•
	TRI	TNAGT5B	45.000	•	1.42	63:90	1.40	1.42	1	BOTTOM SLAB	8.67	1.59	1	TOP ·SLAB	8.67	•



LRFR SUMMARY (LOOKING DOWNSTREAM)

DATE : 10/2014 DATE : 10/2014 C. HOWARD ASSEMBLED BY : M. MILLS DRAWN BY: WMC 7/II REV. 10/1/II REV. 12/17

WSP USA Inc.
434 FAYETTEVILLE STREET
SUITE 1500
RALEIGH, NC 27601
TEL: 1.919.836.4040 LICENSE NO. F-0165

DESIGN ENGINEER OF RECORD:

---0B089699648B4D3... 12/14/2018

Thomas M. Harris

LOAD FACTORS:

DESIGN LOAD RATING FACTORS

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
ЕН	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

**COMMENTS:** 

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

PROJECT NO. 17BP.14.R.79

GRAHAM COUNTY

STATION: 11+53.30 -L-

SHEET 11 OF 11

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE

SEAL 19299

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

LRFR SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS (NON-INTERSTATE TRAFFIC)

SHEET NO. REVISIONS NO. BY: C-11 DATE: DATE:

STD. NO. LRFR5

# STANDARD NOTES

#### DESIGN DATA:

#### MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

---- 30 LBS.PER CU.FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

## DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# <u>ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:</u>

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{1}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 -  $\frac{1}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{1}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 -  $\frac{1}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \( \frac{1}{6} \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH